

Exhibit 16

TECHNICAL MEMORANDUM

Date:

September 24, 2019

To:

Mr. Robert Fitzmaurice Crestview Village, LLC

15 Lake Bellevue Drive, Suite 102 Bellevue, Washington 98005

From:

Kristina Weller, PE/David Baumgarten, LHG

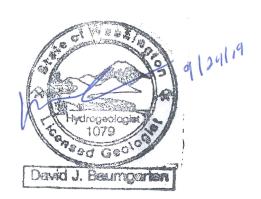
The Riley Group, Inc.

Subject:

Review Comments for Crestview Village II

Infiltration Feasibility

RGI Project No: 2014-169B



This memorandum addresses review comments from Perteet Engineering on the "Drainage Report" for the Crestview II site in Mill Creek, Washington "Review Comments for Crestview II, from Brian Caferro, PE, Perteet Engineering, dated August 22, 2019".

Drainage Report

Comment: Page 17: Ecology still wants to see proof that the infiltration rates are less than 0.30 in/hr via a PIT test, even in till soils. High groundwater table that will not provide required separation can also be used as an infeasibility criteria. High groundwater elevation should be determined by performing groundwater monitoring during the wet season.

Response

The Riley Group (RGI) completed a geotechnical engineering report (GER) in July 2016 for the Crestview Village II site. Six test pits (TP-1 through TP-6) were completed to access soil and groundwater conditions. Subsurface conditions included Vashon-age lodgment till overlain by varying thicknesses of fill, weathered lodgment till, and silt. The depths at which the till was encountered generally ranged from 2 to 5 feet, with the exception of TP-5 in which the till was encountered at a depth of 8 feet below grade. The soils over lying the till in TP-5 included fill soils with concrete and metal debris as well as wood from existing grade to a depth of approximately 5 feet and a soft organic silt from a depth of 5 feet to 8 feet below grade. Fill soils were also found in TP-3 and TP-6.

Light groundwater seepage was noted in test pits TP-2 and TP-6, located on the northern portion of the site, at a depth of approximately 7.5 feet, moist to wet soils were noted at approximately 5.5 feet below grade. A well was installed at the TP-6 location, after the groundwater level stabilized the groundwater level in the TP-6 well was approximately 5.2 feet below grade. Groundwater encountered in TP-2 and TP-6 does not appear to represent a regional water table as it was not encountered in the other test pits at the site. It is likley an isolated zone of groundwater in the more permeable lens in the till units.

RGI's geotechnical report also notes the expectation of a seasonal perched groundwater zone to develop on the unweathered till surface in the winter months. This zone is typically called an "interflow zone". The interflow zone typically ranges in thickness from 6 to 18 inches.

The GER also concludes that based on the soils and groundwater levels encountered at the test pit locations infiltration of stormwater does not appear feasible. Due to the fill soils and shallow depth of the lodgment till "a restrictive layer" the required separation distance (typically 5 feet) for conventional stormwater infiltration facilities (infiltration ponds, infiltration trenches, and infiltration galleries) called for in the Stormwater Management Manual for Western Washington (SWMMWW) could not be met.

The SWMMWW also provides guidance for Low Impact Development stormwater infiltration elements, specifically bioretention. Bioretentiion cells are shallow depressions with a designed planting soil mix and a variety of plant materials, including trees, shrubs, grasses, and/or other herbaceous plants. The SWMMWW manual presents typical design specifications for bioretention cells which includes 1.5 feet of bioretention soil in the base of the cell with a 3-inch layer of course compost overlying the biorentention soil. The SWMMWW manual also shows a "ponding depth" in the cells which the manual notes "varies", typically the ponding depth is a minimum of 18 inches. The elements of the biorentention cell result in a bioretention cell profile of a little more than 3 feet in depth below grade.

The SWMMWW also lists required "separation distances" between the bottom of the bioretention cell and the seasonal high groundwater table, bedrock, or other impervious layer. Given the proposed site development and impervious areas the separation distance from the bottom of the bioretention cell and high groundwater, bedrock, or impervious layer would be 3-feet.

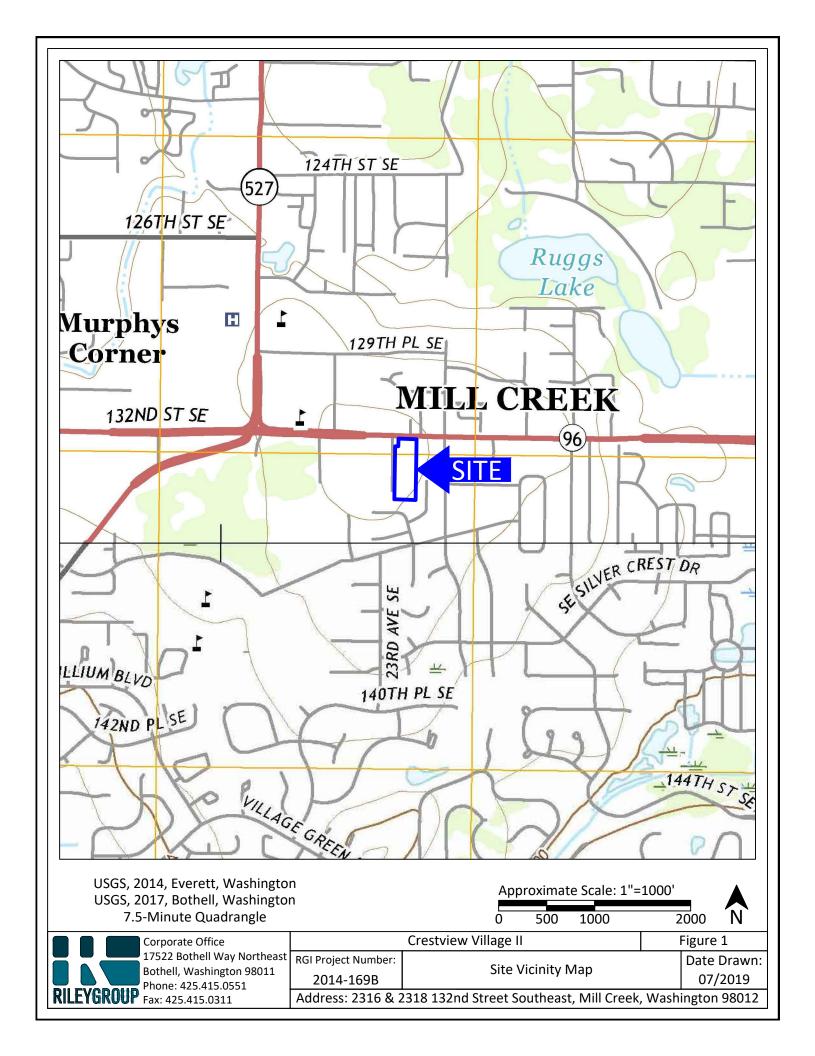
The lodgment till surface (a restrictive layer/impervious layer) is at depths ranging from 2 to 4 feet below grade in test pits TP-1, TP-2, TP-3, TP-4, and TP-6. Given the bioretention cell profile of at least 3-feet described above the required separation distance of 3 feet between the base of the bioretention cell and high groundwater, bedrock, or an impervious layer would not be achieved. In addition the groundwater level measured in the well at the TP-6 location (5.2 feet below grade) would result in less than three feet of separation from the base of a bioretention cell and that groundwater level was measured in July, when we would expect the groundwater level to be at a seasonal low level.

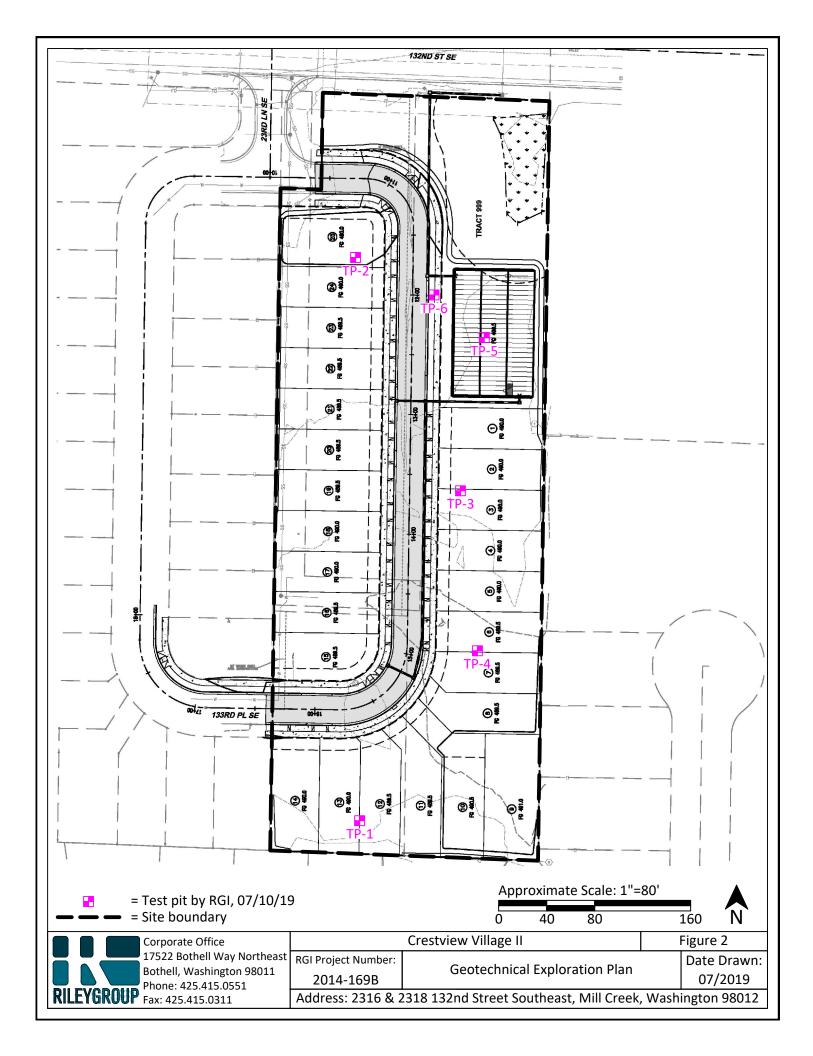


The lodgment till surface in TP-5 was encountered at approximately 8 feet below grade. However organic silt, overlying the till, was encountered at 5 feet below grade. The silt would typically be considered an impervious layer, resulting in less than 3 feet of separation from the base of a bioretention cell to the impervious layer. Furthermore, fill soils were found in TP-5 from grade to the top of the silt unit. The SWMMWW manual only allows infiltration in fill material if the fill is placed and compacted under the direct supervision of a geotechnical engineer or professional civil engineer with geotechnical expertise, and if the measured infiltration rate is at least 8 inches per hour. The fill soils at the site were an uncontrolled fill which contains concrete and metal debris as well as wood

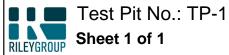
Based on the soils, and depths to seasonal high groundwater, and presence of uncontrolled fill soils, infiltration of stormwater is not feasible at the Crestview Village II site based on the specifications in the SWMMWW for conventional infiltration facilities (infiltration ponds, infiltration trenches, and infiltration galleries) as well as bioretention cells. We understand a detention vault was installed at the adjacent Crestview I site, which has similar soil and groundwater conditions, to manage stormwater runoff.



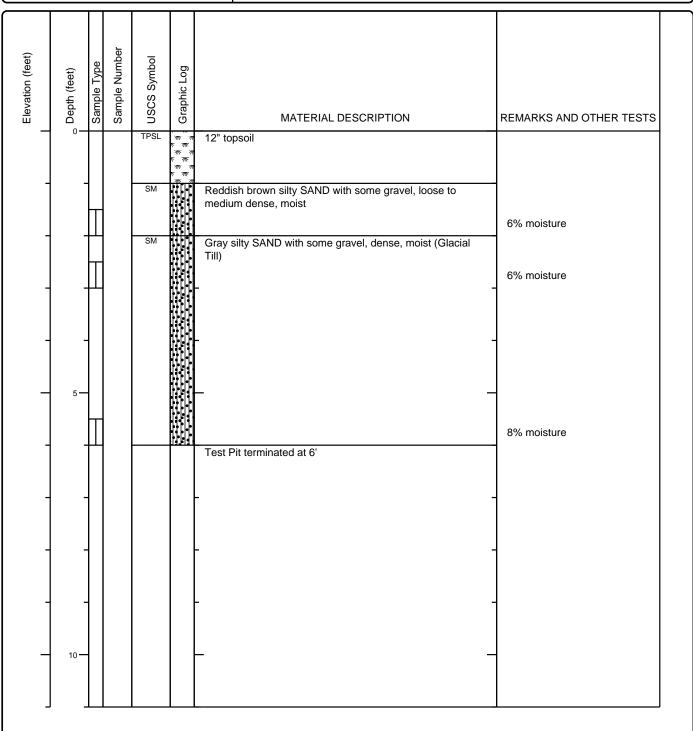




Project Number: 2014-169B Client: Crestview Village, LLC



Date(s) Excavated: 7/10/2019	Logged By ELW	Surface Conditions: Grass			
Excavation Method: Test Pit	Bucket Size: N/A	Total Depth of Excavation: 6 feet bgs			
Excavator Type: Mini Excavator	Excavating Contractor: NW Excavating	Approximate Surface Elevation N/A			
Groundwater Level: Not Encountered	Sampling Method(s) Grab	Compaction Method Bucket			
Test Pit Backfill: Cuttings	Location 2316 and 2318 132nd Street Southeast, Mill Creek, Washington				

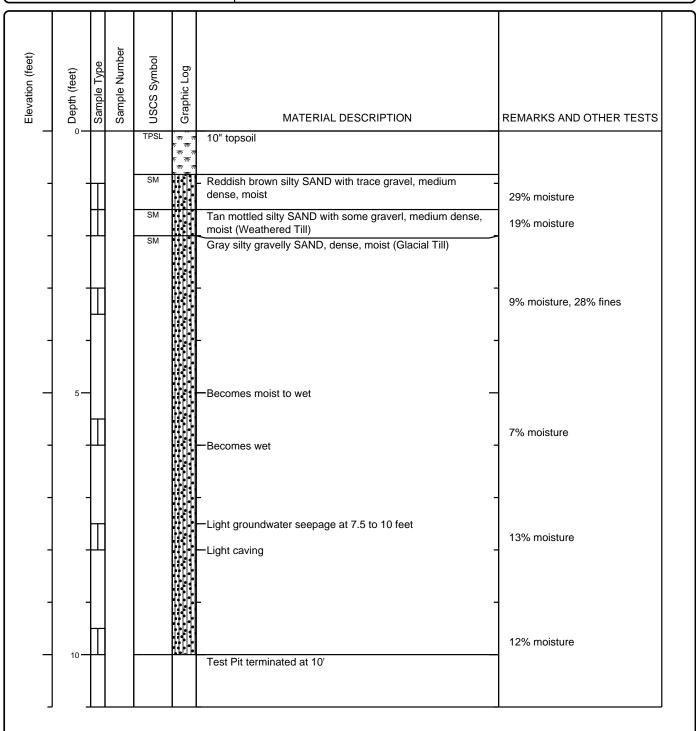


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Test Pit No.: TP-2

Date(s) Excavated: 7/10/2019	Logged By ELW	Surface Conditions: Grass		
Excavation Method: Test Pit	Bucket Size: N/A Total Depth of Excavation: 10 feet bg			
Excavator Type: Mini Excavator	Excavating Contractor: NW Excavating	Approximate Surface Elevation N/A		
Groundwater Level: Not Encountered	Sampling Method(s) Grab	Compaction Method Bucket		
Test Pit Backfill: Cuttings	st, Mill Creek, Washington			

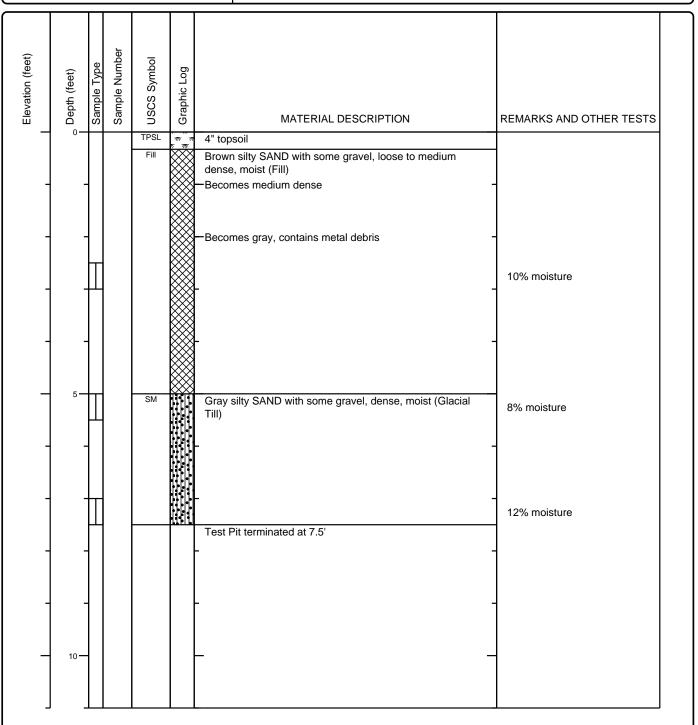


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Test Pit No.: TP-3

Date(s) Excavated: 7/10/2019	Logged By ELW	Surface Conditions: Blackberries		
Excavation Method: Test Pit	Bucket Size: N/A	Total Depth of Excavation: 7.5 feet bgs		
Excavator Type: Mini Excavator	Excavating Contractor: NW Excavating	Approximate Surface Elevation N/A		
Groundwater Level: Not Encountered	Sampling Method(s) Grab	Compaction Method Bucket		
Test Pit Backfill: Cuttings	Location 2316 and 2318 132nd Street Southeast, Mill Creek, Washington			

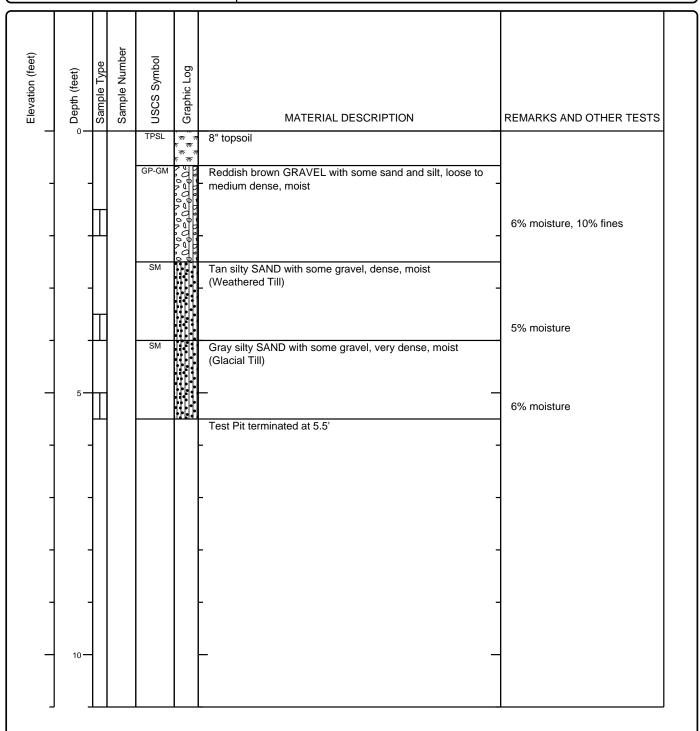


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Test Pit No.: TP-4

Date(s) Excavated: 7/10/2019	Logged By ELW	Surface Conditions: Mixed Brush			
Excavation Method: Test Pit	Bucket Size: N/A	Total Depth of Excavation: 5.5 feet bgs			
Excavator Type: Mini Excavator	Excavating Contractor: NW Excavating	Approximate Surface Elevation N/A			
Groundwater Level: Not Encountered	Sampling Method(s) Grab Compaction Method Bucket				
Test Pit Backfill: Cuttings	Location 2316 and 2318 132nd Street Southeast, Mill Creek, Washington				



Project Number: 2014-169B Client: Crestview Village, LLC



Test Pit No.: TP-5

Date(s) Excavated: 7/10/2019	Logged By ELW	Surface Conditions: Mixed Brush, Grass			
Excavation Method: Test Pit	Bucket Size: N/A Total Depth of Excavation: 9.5 feet b				
Excavator Type: Mini Excavator	Excavating Contractor: NW Excavating	Approximate Surface Elevation N/A			
Groundwater Level: Not Encountered	Sampling Method(s) Grab	Compaction Method Bucket			
Test Pit Backfill: Cuttings	Location 2316 and 2318 132nd Street Southeast, Mill Creek, Washington				

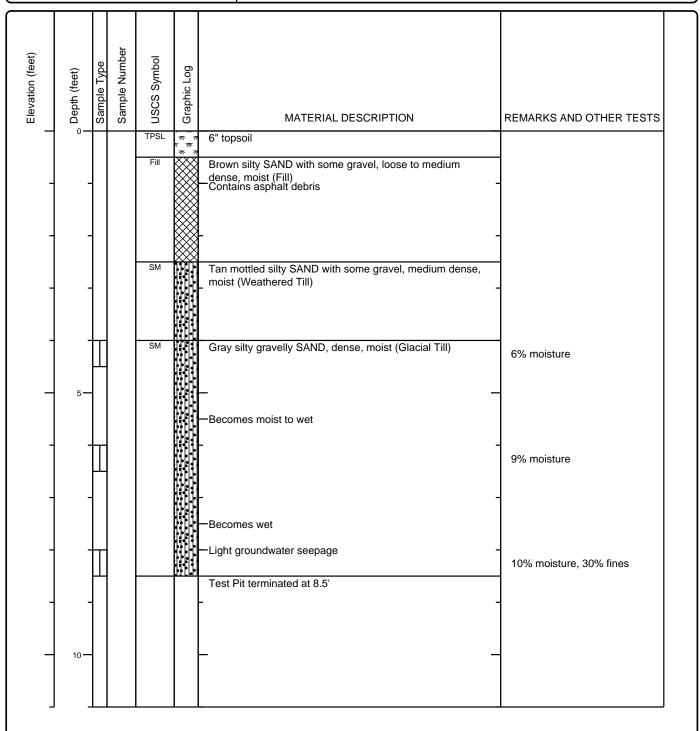
Elevation (feet)	Depth (feet)	Sample Type	Sample Number	USCS Symbol	Graphic Log		
Elev		Sam	Sam			MATERIAL DESCRIPTION	REMARKS AND OTHER TESTS
	0-	П		TPSL Fill		4" topsoil	
				FIII	\bowtie	Brown silty SAND with some gravel, loose, moist	
-	-					-Contains concrete debris	
					\bowtie		
-	-					-	
						Becomes very loose	
-	-					Becomes brown to black, contains organics and wood debris-	
_	_	Ц			\bowtie	_	
		Ш			\bowtie		57% moisture
					\bowtie		
	5—	П		OL		Gray mottled ORGANIC SILT, soft, moist to wet	36% moisture
		П					
-	-					Becomes moist -	
-	-	Н				-	
		Щ					27% moisture
-		Щ		SM		Construction CAND with some arranged damage arraints were	
		Щ		GIVI		Gray silty SAND with some gravel, dense, moist to wet (Glacial Till)	14% moisture
_	_					_	
_		\prod					14% moisture
		П				Test Pit terminated at 9.5'	
_	10 —					-	
-	J _	Ш				<u> </u>	

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Test Pit No.: TP-6

Date(s) Excavated: 7/10/2019	Logged By ELW	Surface Conditions: Mixed Brush, Grass			
Excavation Method: Test Pit	Bucket Size: N/A Total Depth of Excavation: 8.5 feet bg				
Excavator Type: Mini Excavator	Excavating Contractor: NW Excavating	Approximate Surface Elevation N/A			
Groundwater Level: Not Encountered	Sampling Method(s) Grab	Compaction Method Bucket			
Test Pit Backfill: Well Installed	Location 2316 and 2318 132nd Street Southeast, Mill Creek, Washington				



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Key to Logs

Client: C	CIESTA	ew vi	nage,	LLC					
Elevation (feet)		Sample Type Sample Number	USCS Symbol	Graphic Log	MAT		DESCRIPTION		REMARKS AND OTHER TESTS
1		3 4	5	6			7		8
2 Depth 3 Samp show	ation (fee h (feet): l ple Type: vn.	et): Eleva Depth ir : Type o	ation (Manager) at feet be af soil sa	low the gro	ound surface. cted at the depth ii number.	nterval	 Graphic Log: Grap encountered. MATERIAL DESC May include consistext. REMARKS AND C 	hic depic RIPTION stency, m	cool of the subsurface material. tion of the subsurface material : Description of material encountere poisture, color, and other descriptive estates. Comments and observations and made by driller or field personnel
CHEM: C	Chemical Compacti One-dime	tests to ion test ensional	assess	corrosivity			PI: Plasticity Index, pe SA: Sieve analysis (po UC: Unconfined comp WA: Wash sieve (per	ercent pa oressive s	strength test, Qu, in ksf
MATERIA AI	F			<u>S</u> vith Silt (GF	P-GM)		Low plasticity F Silty SAND (SM		.)
TYPICAL	L SAMPI	LER GR	APHIC	SYMBOL	<u>s</u>			OTHER	GRAPHIC SYMBOLS
Bulk S	er sample Sample h-OD Ca s rings		N/		·	2-i	cher Sample nch-OD unlined split oon (SPT) elby Tube (Thin-walled, ed head)	— ▼ Wa Mir	ater level (at time of drilling, ATD) ater level (after waiting) nor change in material properties within a atum erred/gradational contact between strata
gradual. F 2: Descrip	ssification Field descr otions on th	s are bas iptions m hese logs	ay have l apply or	been modifi	ed to reflect results o ecific boring locations	f lab tests	S	•	ive, and actual lithologic changes may be hey are not warranted to be representativ